



Workshop “Eurocodes: background and applications”

Brussels, 18-20 February 2008

General presentation of EUROCODE 7

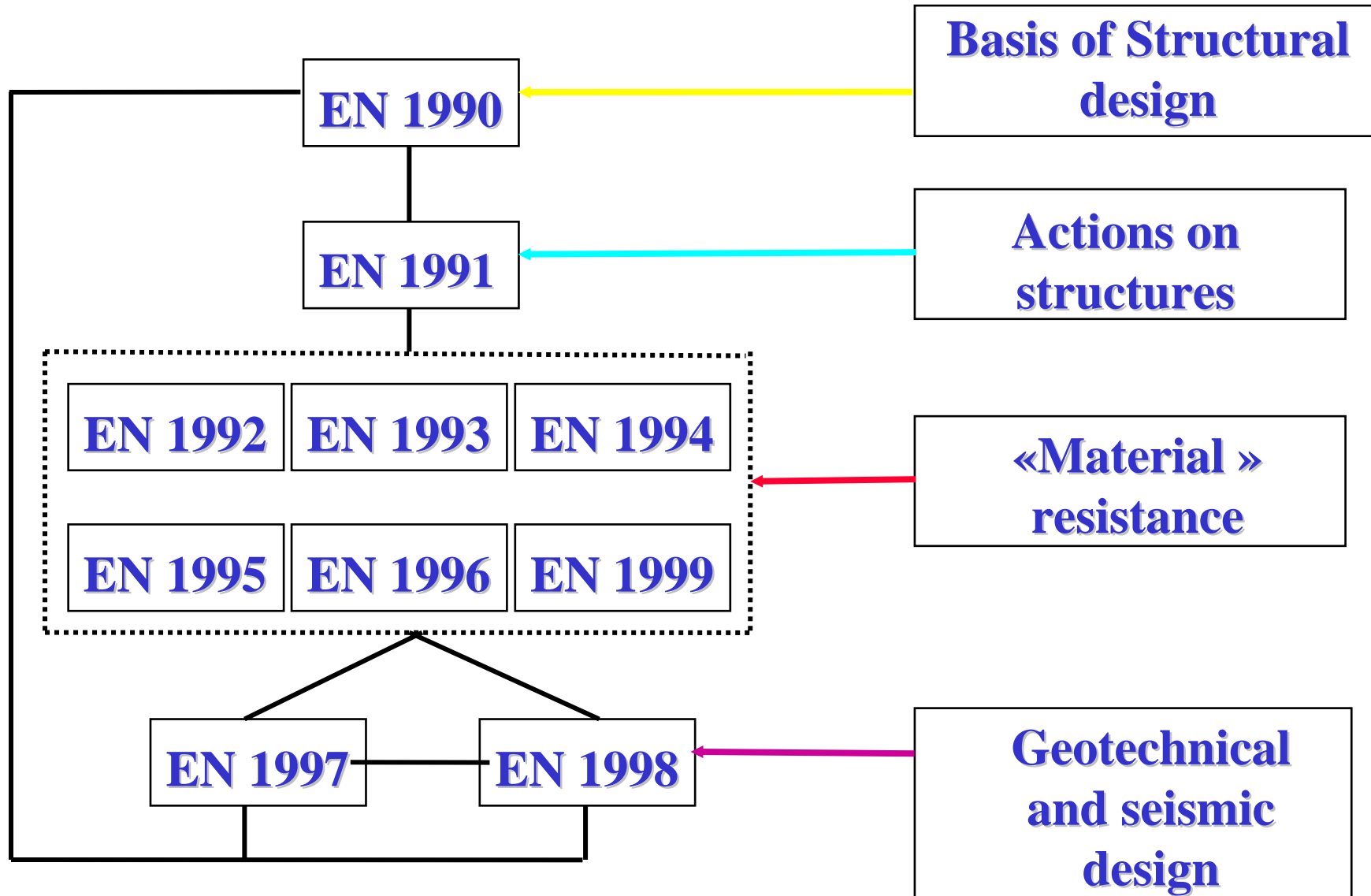
‘Geotechnical design’

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1. Introduction
2. Contents of Eurocode 7 - Parts 1 & 2
3. Some aspects of Eurocode 7-1
 - Characteristic values
 - ULS Design Approaches
 - SLS –Serviceability limit states





Eurocode 7 – Geotechnical design

EN 1997-1 (2004) : Part 1 - General rules

EN 1997-2 (2007) : Part 2 - Ground investigation and testing



2. Contents of Eurocode 7 – Parts 1 & 2

Section 1 General

Section 2 Basis of geotechnical design

Section 3 Geotechnical data

Section 4 Supervision of construction, monitoring and maintenance

Section 5 Fill, dewatering, ground improvement and reinforcement

EUROPEAN STANDARD **EN 1997-1**
NORME EUROPÉENNE
EUROPÄISCHE NORM November 2004

ICS 91.120.20 Supersedes ENV 1997-1:1994

English version

Eurocode 7: Geotechnical design - Part 1: General rules


Eurocode 7: Calcul géotechnique - Partie 1: Règles générales Eurocode 7: Entwurf, Berechnung und Bemessung in der Geotechnik - Teil 1: Allgemeine Regeln

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Section 6 **Spread foundations**

Section 7 **Pile foundations**

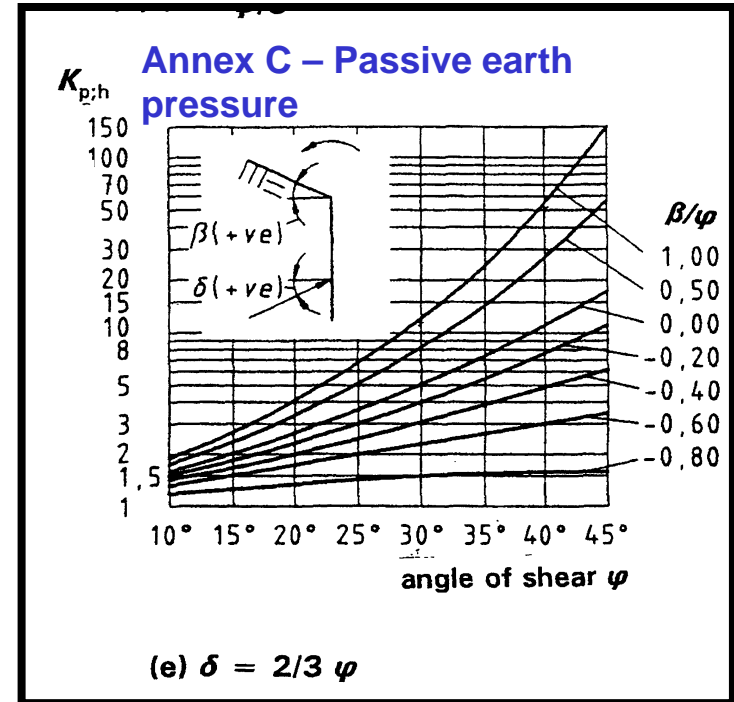
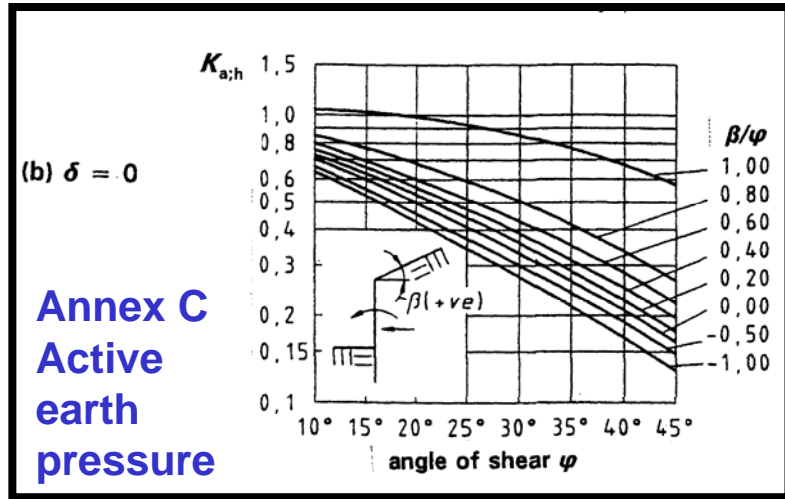
Section 8 Anchorages

Section 9 **Retaining structures**

Section 10 Hydraulic failure

Section 11 Site stability

Section 12 Embankments



Annexes D & E : Bearing capacity of foundations

$$R/A' = c' \times N_c \times b_c \times s_c \times i_c +$$

$$q' \times N_q \times b_q \times s_q \times i_q +$$

$$0,5 \times \gamma' \times B' \times N_\gamma \times b_\gamma \times s_\gamma \times i_\gamma$$

$$R/A' = \sigma_{v0} + k \times p_{le}^*$$

Annex F : Settlement of foundations

$$s = p \times b \times f / E_m$$



Part 2 (EN 1997-2): Geotechnical design - Ground investigation and testing

Laboratory and field tests :

- * essential requirements for the equipment and tests procedures
- * essential requirements for the reporting and the presentation of results
- * interpretation of test results and derived values

They are NOT test standards → see TC 341



- Section 1 General
- Section 2 Planning and reporting of ground investigations
- Section 3 Drilling, sampling and gw measurements
- Section 4 Field tests in soils and rocks
- Section 5 Laboratory tests on soils and rocks
- Section 6 Ground investigation report

> Also a number of **Informative annexes**


<p>EUROPEAN STANDARD NORME EUROPÉENNE EUROPÄISCHE NORM</p> <hr/> <p>ICS 91.050.01; 91.120.20</p>	<p>EN 1997-2</p> <p>March 2007</p> <hr/> <p>Supersedes ENV 1997-2:1999, ENV 1997-3:1999</p> <p>English Version</p> <p>Eurocode 7 - Geotechnical design - Part 2: Ground investigation and testing</p> <p>Eurocode 7 - Calcul géotechnique - Partie 2: Reconnaissance des terrains et essais</p> <p>Eurocode 7 - Entwurf, Berechnung und Bemessung in der Geotechnik - Teil 2: Erkundung und Untersuchung des Baugrunds</p>
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3. Some aspects of Eurocode 7-1

Characteristic values and design values

ULS Design Approaches

SLS and deformations of structures

Type of test

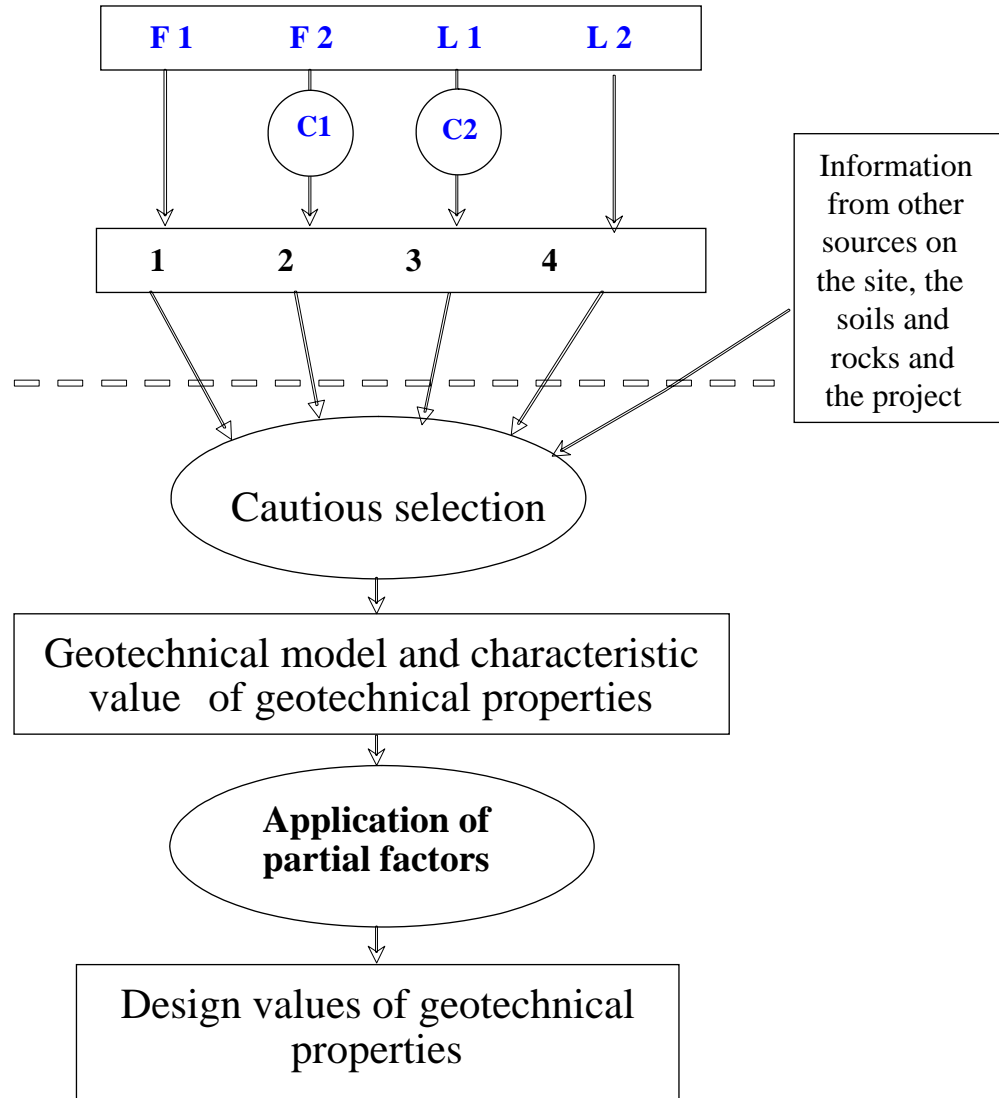
F= field L= laboratory

Correlations

Test results and derived values

EN 1997 -2

EN 1997 -1





Characteristic value of geotechnical parameters

P The **characteristic value** of a geotechnical parameter shall be selected as a cautious estimate of the value affecting the occurrence of the limit state.

If statistical methods are used, the **characteristic value** should be derived such that the calculated probability of a worse value governing the occurrence of the limit state under consideration is not greater than 5%.



Design values of geotechnical parameters

Design value of a parameter : $X_d = X_k / \gamma_M$

Design values of actions and resistances

fulfilling for STR/GEO ULS : $E_d \leq R_d$

$$E_d = E \{ \gamma_F \cdot F_k \} \quad \text{and} \quad R_d = R \{ X_k / \gamma_M \}$$

(= “at the source”, MFA)

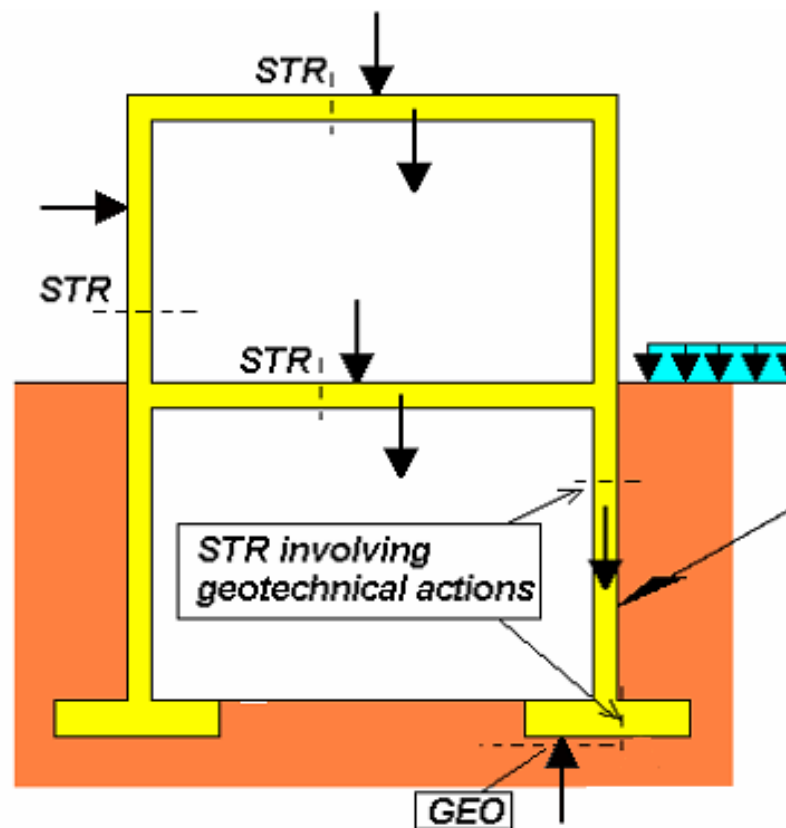
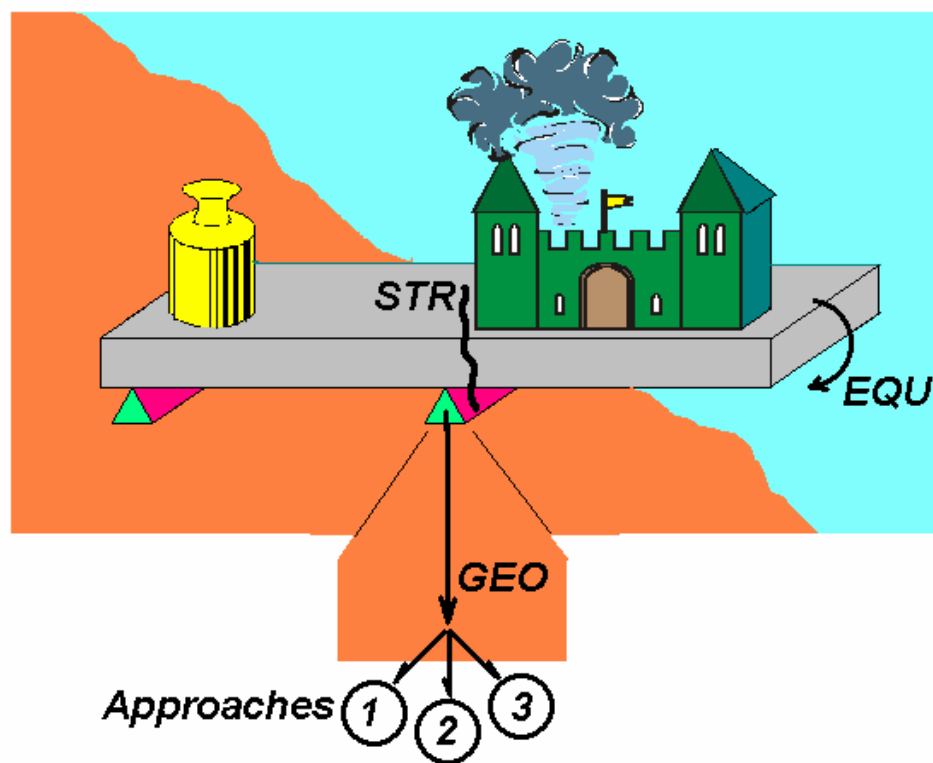
or $E_d = \gamma_E \cdot E \{ F_k \} \quad \text{and} \quad R_d = R \{ X_k \} / \gamma_R$
(RFA)



Ultimate limit states – Eurocode 7-1

- **EQU** : loss of equilibrium of the structure
- **STR** : internal failure or excessive deformation of the structure or structural elements
- **GEO** : failure or excessive deformation of the ground
- **UPL** : loss of equilibrium due to uplift by water pressure (buoyancy) or other vertical actions
- **HYD** : hydraulic heave, internal erosion and piping caused by hydraulic gradients

EN1990 - Ultimate limit states EQU and STR/GEO



$$E_d < R_d$$

J.A Calgaro



Approach	Combinations	Action (γ_F)	Symbol	Set A1	Set A2
1	A1 “+” M1 “+” R1	Permanent Unfavourable	γ_G	1,35	1,00
	& A2 “+” M2 “+” R1				
2	<u>Or</u> A2 “+” M1 or M2 “+” R4	Variable Unfavourable	γ_Q	1,50	1,30
	A1 “+” M1 “+” R2				
3	A1 or A2 “+” M2 “+” R3	Favourable	γ_Q	0	0

Soil parameter (γ_M)	Symbol	Set M1	Set M2
Angle of shearing resistance	$\gamma_{\phi'}$	1,00	1,25
Effective cohesion	$\gamma_{c'}$	1,00	1,25
Undrained shear strength	γ_{cu}	1,00	1,40
Unconfined strength	γ_{qu}	1,00	1,40
Weight density	γ_γ	1,00	1,00

Resistance (γ_R)	Symbol	Set R1	Set R2	Set R3
Bearing Portance	γ_{Rv}	1,00	1,4	1,00
Sliding	γ_{Rh}	1,00	1,1	1,00

← γ_R for Spread foundations



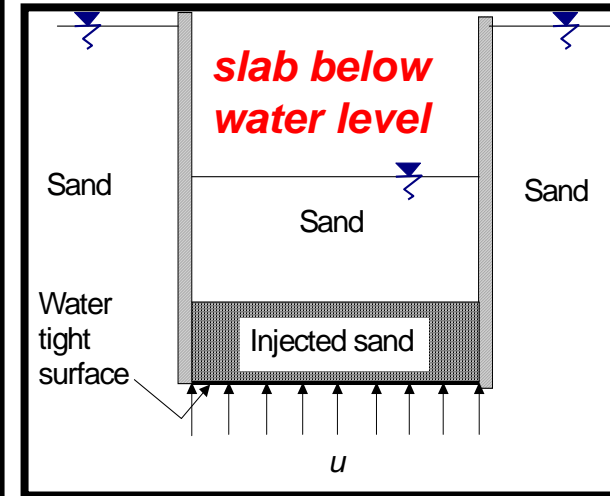
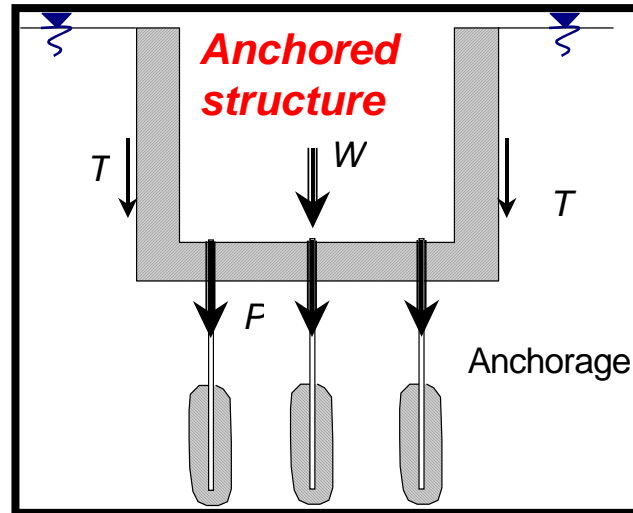
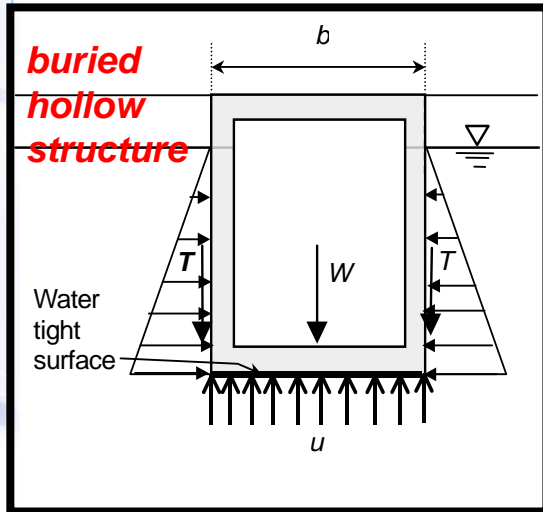
STR/GEO : accidental situations

Actions : all values of γ_F (and γ_M) = 1.0

Resistances :

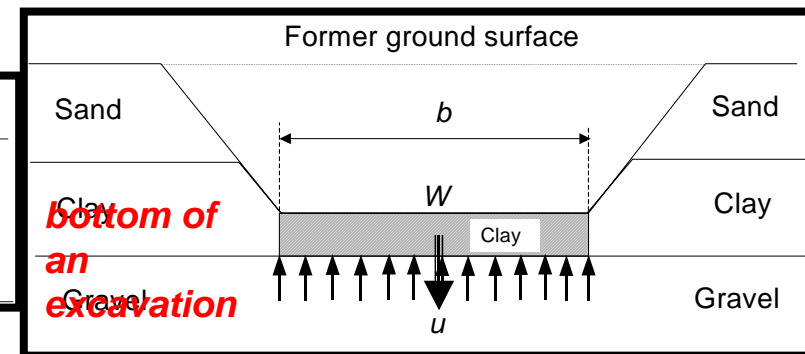
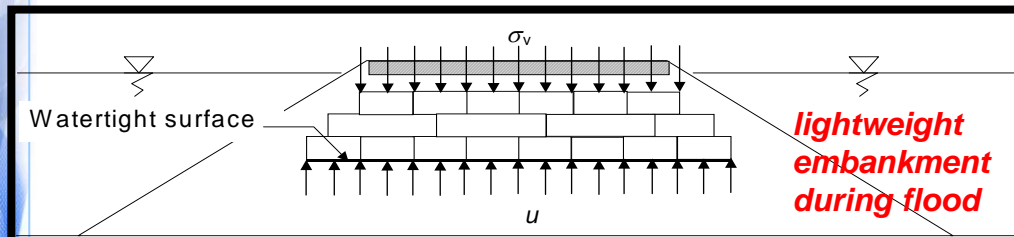
all values of γ_R (and γ_M) depend
on the particular accident

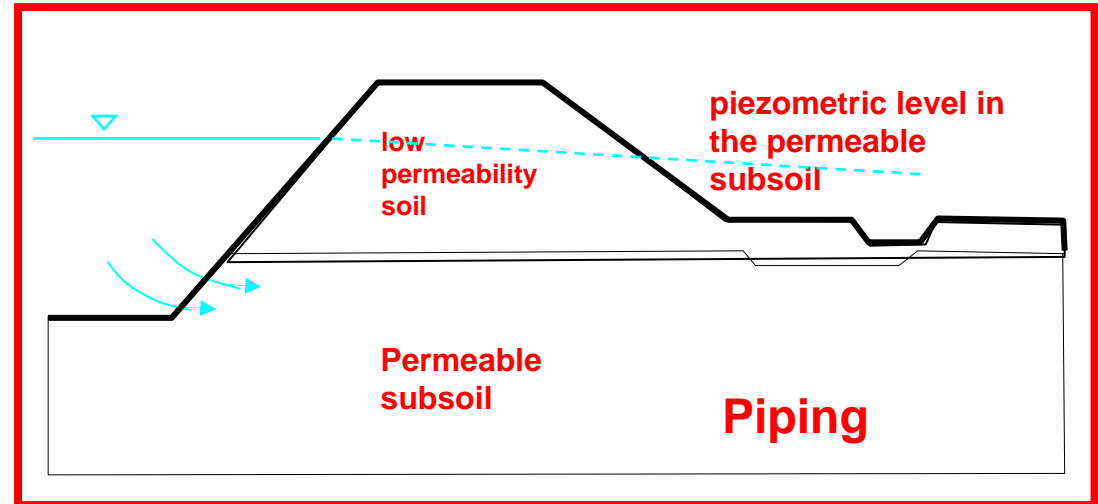
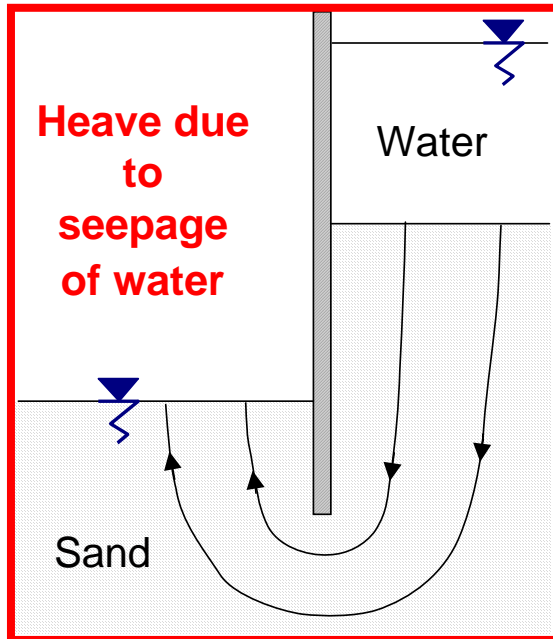
Seismic situations: see Eurocode 8-5



Examples of situations where uplift might be critical

$$G_{dst;d} + Q_{dst;d} \leq G_{stb;d} + R_d$$





$$u_{dst;d} \leq \sigma_{stb;d}$$

$$\Delta u_{dst;d} \leq \sigma'_{stb;d}$$

Example of situation where heave or piping might be critical



Ultimate limit states of static equilibrium (EQU) :

$$E_{d,dst} \leq E_{d,stb}$$

Ultimate limit states of resistance (STR/GEO) :

$$E_d \leq R_d$$

Ultimate limit state of uplift (UPL) :

$$G_{dst;d} + Q_{dst;d} \leq G_{stb;d} + R_d$$

Ultimate limit state of hydraulic failure (HYD) :

$$u_{dst;d} \leq \sigma_{stb;d} \quad \text{or} \quad S_{dst;d} \leq G'_{stb;d}$$



EN1990 - Serviceability limit states SLS

Verifications :

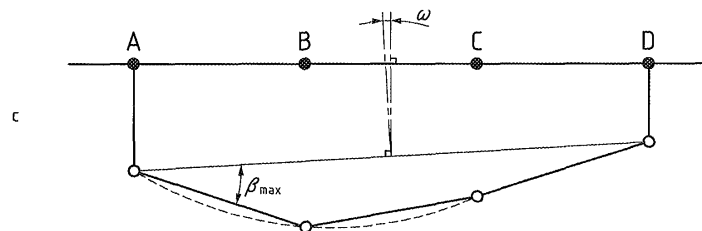
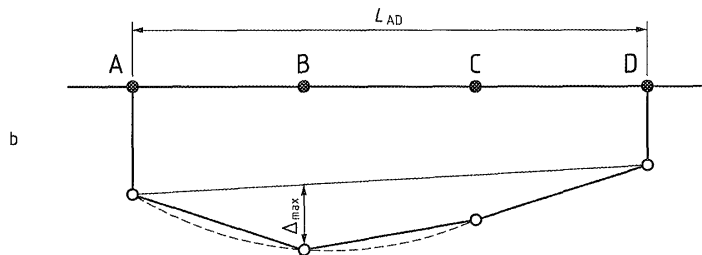
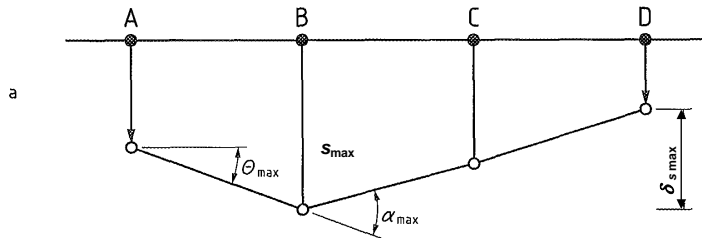
$$E_d \leq C_d$$

C_d = limiting design value of the relevant serviceability criterion

E_d = design value of the effects of actions specified in the serviceability criterion, determined on the basis of the relevant combination

$$\text{all } \gamma_F \text{ and } \gamma_M = 1.0$$

Movements and deformations of structures



settlement s , differential settlement δs , rotation θ and angular strain α

relative deflection Δ and deflection ratio Δ/L

ω and relative rotation (angular distortion) β

(after Burland and Wroth, 1975)



Eurocode 7 :

- a tool to help European geotechnical engineers speak the **same language**
- a necessary tool for the **dialogue** between geotechnical engineers and structural engineers

Eurocode 7 helps **promoting research**

- **it stimulates questions** on present geotechnical practice from ground investigation to design models



and to really conclude :

It should be considered that knowledge of the ground conditions depends on the extent and quality of the geotechnical investigations. Such **knowledge and the control of workmanship** are usually more significant to fulfilling the **fundamental requirements** than is precision in the calculation models and partial factors.



Thank you for your attention !