

service d'Études techniques des routes et autoroutes



Brussels, 18-20 February 2008 – Dissemination of information workshop

General presentation of Eurocode 4







EUROCODE 4



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EUROCODE 4 : Design of composite steel and concrete structures

EN 1994-1-1: general rules and rules for <u>buildings</u>

EN 1994-1-2: structural fire design

EN 1994-2: general rules and rules for bridges

The general rules valid for bridges from part 1-1 are repeated in part 2 to get a self sufficient document.



Annex C

EN 1994-1-1



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Forward		
Section 1	General	
Section 2	Basis of design	Common to all EC
Section 3	Material	
Section 4	Durability	Lavout
Section 5	Structural analysis	Layout common to all EC
Section 6	ULS	Common to an LC
Section 7	SLS	
Section 8	Composite joints in frames for buildings	
Section 9	Composite slabs for buildings	
Annex A	(informative) Stiffness of joint in buildings	
Annex B	(informative) Standard tests	

(informative) Shrinkage of concrete for buildings



EN 1994-2



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Forward

Section 1 General

Section 2 Basis of design

Section 3 Material

Section 4 Durability

Section 5 Structural analysis

Section 6 ULS

Section 7 SLS

Section 8 Precast concrete slabs in bridges

Section 9 Composite plates in bridges

Annex C Headed studs that cause splitting in the slab

thickness



Rules for drafting



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The paragraphs specific to buildings are put at the end to be easily modified.

6.4 Lateral-torsional buckling of composite beams

6.4.1 General

EN 1994-1-1

- (1) A steel flange that is attached to a concrete or composite slab by shear connection in accordance with 6.6 may be assumed to be laterally stable, provided that lateral instability of the concrete slab is prevented.
- (2) All other steel flanges in compression should be checked for lateral stability.
- (3) The methods in EN 1993-1-1, 6.3.2.1-6.3.2.3 and, more generally, 6.3.4 are applicable to the steel section on the basis of the cross-sectional forces on the composite section, taking into account effects of sequence of construction in accordance with 5.4.2.4. The lateral and elastic torsional restraint at the level of the shear connection to the concrete slab may be taken into account.
- (4) For composite beams in buildings with cross-sections in Class 1, 2 or 3 and of uniform structural steel section, the method given in 6.4.2 may be used.
- 6.4.2 Verification of lateral-torsional buckling of continuous composite beams with cross-sections in Class 1, 2 and 3 for buildings



Rules for drafting



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The paragraphs specific to bridges are added at the end of the clauses.

6.4 Lateral-torsional buckling of composite beams

EN 1994-2

6.4.1 General

- (1) A steel flange that is attached to a concrete or composite slab by shear connection in accordance with 6.6 may be assumed to be laterally stable, provided that lateral instability of the concrete slab is prevented.
- (2) All other steel flanges in compression should be checked for lateral stability.
- (3) The methods in EN 1993-1-1: 2005, 6.3.2.1-6.3.2.3 and, more generally, 6.3.4 are applicable to the steel section on the basis of the cross-sectional forces on the composite section, taking into account effects of sequence of construction in accordance with 5.4.2.4. The lateral and elastic torsional restraint at the level of the shear connection to the concrete slab may be taken into account.

6.4.2 Beams in bridges with uniform cross-sections in Class 1, 2 or 3



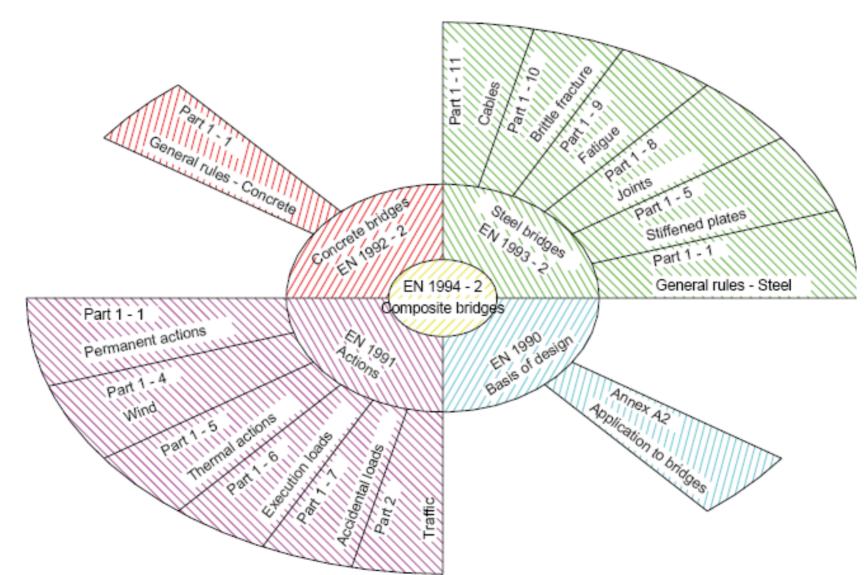
Rules for drafting



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Avoid cascades of references





Scope of EN 1994-1-1



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Composite members

Composite beams
Composite columns

Composite slabs

Composite joints



Composite beams

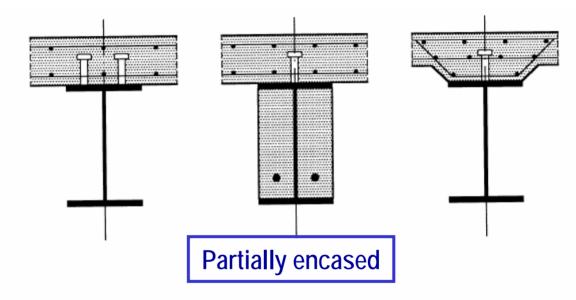


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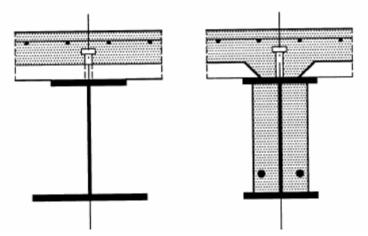
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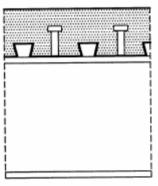
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Solid slab



Composite slab



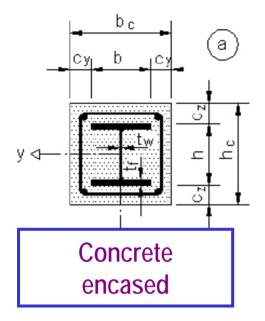


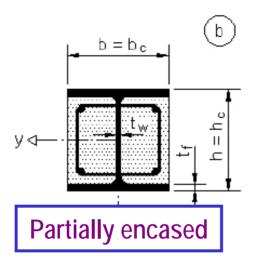


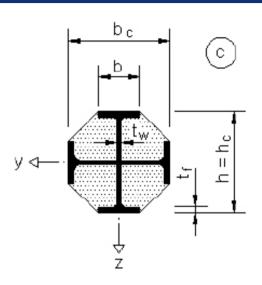
Composite columns

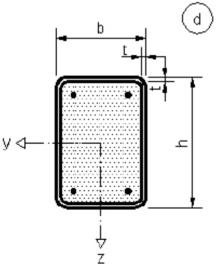
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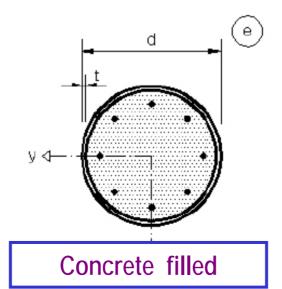
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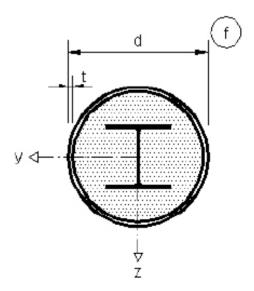












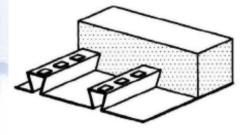


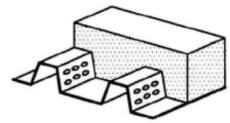
Composite slabs

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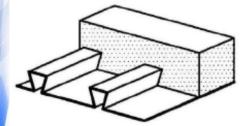
1

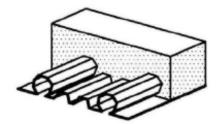
a) mechanical interlock



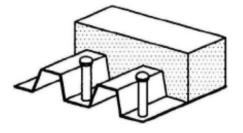




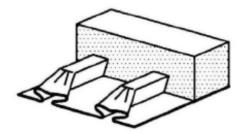




c) end anchorage by through - deck welded studs



d) end anchorage by deformation of the ribs





Composite joints

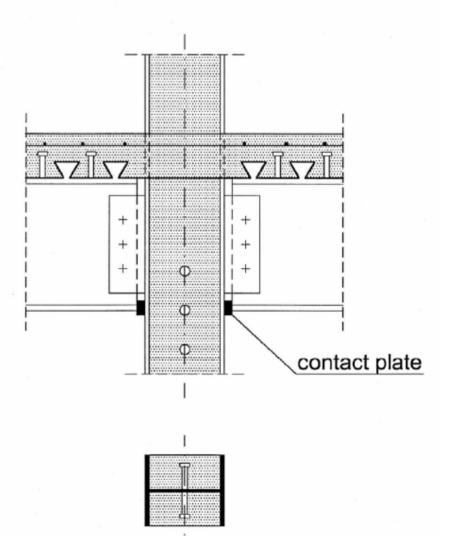


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single-sided configuration

double-sided configuration





Scope of EN 1994-2

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Composite bridges

I girders

Box sections

Cable stayed bridges not fully covered

Composite members

Filler beam decks

Tension members

Composite plates



Composite bridges

Sétra

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Composite members



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Filler beam decks

Sétra

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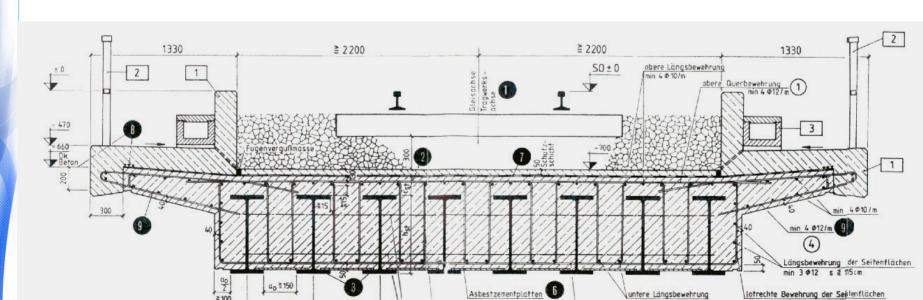
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longitudinal





Tension members



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Composite plates



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Coordination EC4-EC3: materials



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	EC4	EC3
Grade of steel	S 235 – S 460	S 235 – S 460 + EN 1993-1-12 (S 690)
Coefficient of expansion	10 10 ⁻⁶ equal for steel and concrete	12 10 ⁻⁶



Coordination EC4-EC2: materials



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	EC4	EC2
Concrete strength	C20 - C60	C12 – C90
shrinkage	As in EC2 or annex C (3,25x10 ⁻⁴ in dry environment)	
Modulus of elasticity	210 000 (as in EC3) equal for steel and reinforcement	200 000



Coordination EC4-EC3: design rules

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	EC4	EC3
Effective width	Slab : EC4 (same at SLS/ULS) steel flange : EN 1993-1-5	EN 1993-1-5 (SLS ≠ ULS)

Coordination EC4-EC2 : design rules

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	,	EC4	EC2
	Design value	$f_{cd} = f_{ck} / \gamma_C$ 0.85 is a calibration factor of $M_{pl,Rd}$	$f_{cd} = \alpha_{cc} f_{ck} / \gamma_{c}$
	Effective width	β_{2} : $L_{e} = 0.25 (L_{1} + L_{2})$ β_{1} : $L_{e} = 0.85L_{1}$ β_{1} : $L_{e} = 0.70L_{2}$	$l_0 = 0.85 \ l_1$ $l_0 = 0.15 \ (l_1 + l_2)$ $l_0 = 0.7 \ l_2$ $l_0 = 0.15 \ l_2 + l_3$
1	Shear	Vertical shear resistance of the cracked slab in EC2 has been modified	